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| PROJECT:<br>BOYLE MI |                  | SHEET NO.<br>1/10 |
| BY:<br>CUF           | DATE:<br>5/31/18 | JOB NO.<br>17147  |

SEE SHEETS S1.0 S1.1 S2.1 S2.2 S2.3 S3.1 S3.2 FOR REFERENCE  
NUMBERS  
- ATTACHED

SHEET S1.0

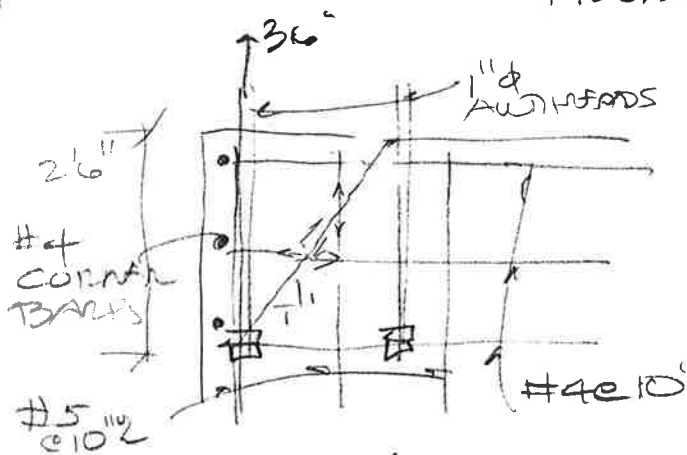
- 1) SEE SHORING DESIGN BY OTHERS
- 2) SEE ADD'D STEEL SPECIAL INSPECTION
- 3) LEGIBLE TABLES PROVIDED

SHEET S1.1

- 1) SEE REVISED DETAIL  $\left(\frac{3}{S1.1}\right)$  FOR 2' MAX
- 2) CHANGE EMBEDMENTS TO 30"  
USE SHEAR FRICTION FOR PULLOUT STRENGTH

ACTUAL LOAD =  $\frac{1.6}{2} \left(\frac{9}{2}\right) = 3.6 \text{ k (ASD)}$   
TENSION  
FROM 14/30 CALCS

$V_u = 3.6(1.6) = 5.8 \text{ k}$



$\phi V_n = A_v f_y (\mu \sin \alpha_f + \cos \alpha_f)_{.75}$

$(13 + 1.20) 60 (1.4 \sin 45 + \cos 45) = 52 (1.75) = 39 \text{ k}$   
#5 #4

O.K.

$A_c = 10(42) = 420 \text{ in}^2$



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- 3) SEE LEDGEBASE DETAIL W/ HARDWARES (2) #3
- 4) MST #8 REMOVED - NOT USED
- 5) NOTE ADDED - TOP & BOTTOM R'S ARE AT PARALLEL EDGE
- 6) USE SB  $\frac{3}{8}$  X 24 ANCHOR BOLT FOR HDUS
- 7) SEE ADDED DETAIL  $\frac{2}{S11}$

SHEET S 2.1

1)



$$F_p = 194 \left( \frac{18}{14} \right) \quad K1 = 1.54 K1$$

$$W = 10 \frac{1}{2} (15) \frac{1}{2} = 1.56 k$$

$$F_p = 1.56 (1.54) = 1.30 k$$

USE HG410 @ 16" @ OK

MODIFY  $\frac{FL7}{S31L}$

$$C_{AO} \frac{3}{4} \phi AB = 830 / b$$

$$830 \frac{1}{2} (116) = 1.66 k > 1.3 k$$

$\frac{3}{4}$ " X 4 X 8 @ 2' 0" OK

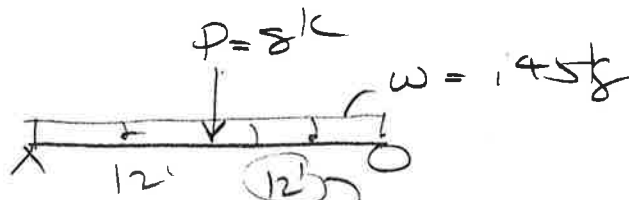
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- 2) SPIRAL STAIR ELIMINATED
- 3) CONCRETE ABOVE FIRST FLOOR - ELIMINATED
- 4) SHOULD BE  $\frac{FL6}{S312}$  SIM
- 5) HO, REMOVE SIM
- 6) SEE ADDED DETAIL  $\frac{F13}{S313}$
- 6a) THIS CONDITION IS DELETED
- 7) THIS FOOTING IS ATTACHED LOW ON THE WALL, THIS WILL NOT ADD ADDITIONAL LOADS TO THE WALL
- 8) SEE ADDED DETAIL  $\frac{1}{S112}$
- 9) SEE ADD DETAIL  $\frac{2}{S112}$
- 10) SEE DETAIL  $\frac{FB}{S313}$  NEW
- 11) 4" CONC SUB CAULFOOT
- 12) SEE STRUCTURE DESIGN BY OTHERS
- 13) SEE ARCHITECTURAL DRAWINGS SECTION A33, FULL ELEVATIONS

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STREET 52.2

1) BEAM B3 SHOULD BE 5 1/2" x 24" GLULAM



$$R_0 = \frac{\sqrt{178(12)^2}}{55^2} = 13.0$$

$$F_{BE} = \frac{16(1.80000)}{13.0^2} = 6497 \text{ psi}$$

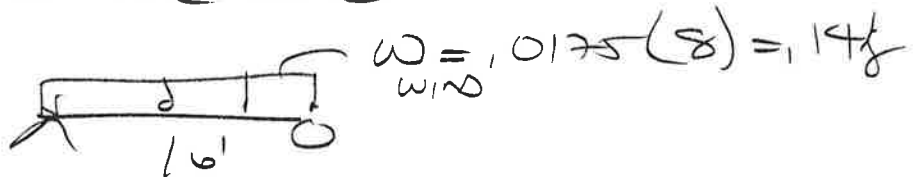
$$F_{BE} = \frac{6497}{2760} = 2.35$$

$$M = \left( \frac{45(24)^2}{8} + \frac{8(24)}{4} \right) \frac{12}{176} = 1005$$

$$C_L = \frac{1 + 2.35}{1.9} - \sqrt{\left( \frac{1 + 2.35}{1.9} \right)^2 - \frac{2.35}{175}} = 1.96$$

$$S_{FE} = \frac{1005}{2.4(1.15)} = 364 \text{ in}^3$$

HORIZONTAL LOAD



$$M = \frac{1.4(16)^2}{8} \frac{12}{12} = 54 \text{ k-in}$$

$$S_{FE} = \frac{54}{1.55(1.6)} = 18.2 \text{ in}^3$$

5 1/2" x 24" GLULAM  
OK

2) GOLF MOVE & HEW LOCATIONS ADDED

3) HANGERS ADDED

4) SEE NEW DETAIL  $\frac{RS}{S313}$

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5) THE BOTTOM OF BEAM IS FLUSH  
W/ BOTTOM OF FLOOR JOISTS - SEE  
ARCH FOR STEP UP INTO ATTIC  
AREA

6) REVISED  $\frac{12}{S312}$  W/ S HEALWAY ABOVE

7) SEE  $\frac{R6}{S313}$

8) SEE NEW  $\frac{R5}{S313}$

9) SEE PLAN FOR HORIZ. STRAPS TO  
TIE CORNERS.

SHEET S23

1) SEE ADDED STAIR DETAILS

2) SEE STRAPS 18g/30 & 18b/30 OF CHAINS  
FOR KEY OF BEAMS

3) SEE NEW DETAIL  $\frac{EL14}{S313}$

4) ADDRESSED PARALLEL

5) ADDRESSED PARALLEL

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6) SEE DETAIL  $\frac{FL15}{S313}$  &  $\frac{FL16}{S313}$

7) SEE DETAIL  $\frac{F}{S112}$

8) HOTO ON PLAN

9) ADD STAFFMAN R'S - SEE DETAIL  $\frac{FS}{S311}$

10) SEE ADDED LENGTH CALCULS

11) ELEVATOR WILL RESIST LATERAL LOADS IN THE NW-SE DIRECTION ONLY

$$V_{SEIS} = \frac{.94}{5}(1.4) W = 1.13 W$$

$$W = (.015 + .015 + .015) 11(22) = 10.9 k$$

$$V_{SEIS} = 1.13(10.9) = 12.3 k \ll 4.0 k \text{ FOR WIND}$$

SEE ORIG. CALCS ON SHEET 29/30 - SHAFT

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12) THE 4" CONCRETE IS ELIMATED - ADD ADDITIONAL 1 1/8" HEIGHT TO FLOOR

13) SEE PLANS & DETAILS  $\frac{F15}{S313}$  &  $\frac{F116}{S313}$

14) SEE REVISED PLANS & DETAILS

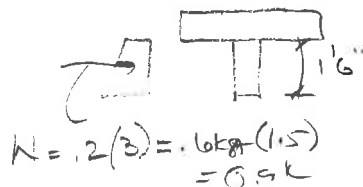
15) THE WSW IN PLANE w/ OUTSIDE OF RETAINING WALL & OUTSIDE OF WSW  
SEE ADDED DETAIL  $\frac{F15}{S313}$

16) CHANGED TO H202 - SEE PLANS

17) HUC 6/2 FRAMED DIRECTLY INTO EXTENSION WALL - SEE PLAN

SHEET S311

1) SINCE SOIL IS SMC CLASS C & PER. ASCE 7-10 SEC 11.8.1.2 SEISMIC SURCHARGE IS NOT REQUIRED  
THE 0.3 FRICTION IS IN THE ULTIMATE CONDITION, I USE 0.40 ASD AND USED A FACTOR OF SAFETY OF 1.5  
9' IS MAX HT ALLOWED - THE TABLE ON 22/30 - SI PST KFO (I USED 550 psf)



$$M_U = 1.0(9)(75)(12) = 13.0k'$$

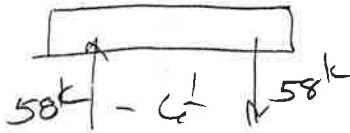
$$R_U = \frac{13}{17(12)5^2} = 1098(lb)$$

$$A_s = \frac{48(1.15)12(5)0.00333}{200} = 1.07 \text{ in}^2/\text{ft}$$

#4 @ 24" oc

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- 2) REVISED DETAIL
- 3) REVISIONS TO SCHED
- 4) SEE MODIFIED DETAIL - FORCES BETWEEN PILES ONLY BY THIS



$$M = 58(94)12 = 4176$$

$$R_u = \frac{4176(1.4)}{19(94)12^2} = 154 \text{ ksi}$$

$$A_s = .01(94)12 = 11.5 \text{ in}^2 \quad \#8 @ 6" \text{ O.C.}$$

CHANGE SEE  $\frac{F7}{S3.1}$

- 5) SEE GEOTECH REPORT PAGE 27/30 & CALCS 34/30 - NO SEISMIC SURCHARGE AS STATED IN 1)

- 6) SEE MODIFIED DETAIL  $\frac{F7}{S3.1}$

- 7) SEE 4)

- 8) SEE GEOTECH 27/30 - SURCHARGE = 2500 psf &  $K_{H0} = 46 \text{ pd ON SHAFT}$  & CALCS ON 26/30, 27/30 & 24/30



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- 9) BOWTIE IS HELD TOGETHER BECAUSE OF DRAINS (A3.3) NOT REAR  
SEE A3.3 FOR STYROFOAM.
- 10) SEE REVISED DETAIL
- 11) SEE REVISED DETAIL

SHEET S32

- 1) SEE REVISED DETAIL
- 2) 2X4 FLAT BLOCKING & SHEATHING - REVISED DETAIL
- 3) SEE RESPONSE 1) SHEET S2.1
- 4) CAPACITY OF UP  $3/4" \phi @ 2'0" L$  FOR 33'  
 $= 1.68(14)1.6 = 43K - OK$
- CAPACITY OF ABT FOR 33' @ 2'0"  
 $= .420(14) = 6.7K - OK > 6.5K$   
 4/30  
 CALCS
- SEE REVISED CONTINUOUS CONCRETE FACE
- 5) SEE REVISED DETAIL
- 6) SEE REVISED DETAIL
- 7) SEE REVISED DETAIL
- 8) SEE REVISED DETAIL

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- 9) SEE REVISED DETAIL
- 10) SEE REVISED DETAIL
- 11) SEE REVISED DETAIL
- 12) SEE CALC ON SHEETS 22 - 1) PREVIOUSLY
- 13) SEE REVISED DETAIL
- 14) SEE CALCS ON CR 6, CALCS 19/30
- 15) LATERAL LOAD FROM DECK WILL  
BE RESISTED BY RESIDENCE  
SEE DETAIL FLUG AND PART  
S313
- 16) MODIFIED DETAIL
- 17) HI REVISED
- 18) ADD A35 - REVISED
- 19) REVISED TO "RBC"
- 20) REVISE - edge 4" E
- 21) REVISE - 2' 0"
- 22) REVISED BY HI
- 23) REVISED - BUNDAM NAILITE